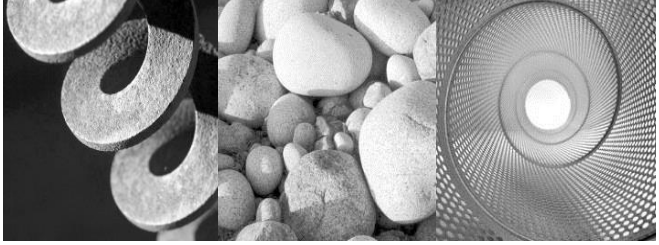


GEI Consultants

Consulting
Engineers and
Scientists

Regulation Compliance Report

Closure Plan

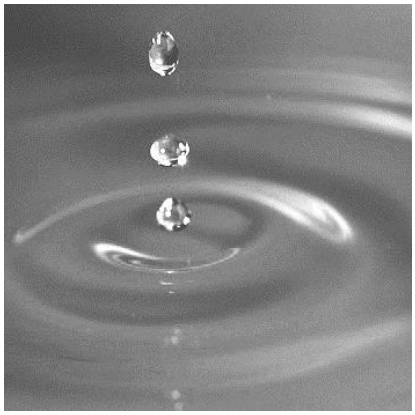
Weston Disposal Site No. 3 Landfill
Town of Knowlton, Marathon County, Wisconsin

Submitted to:
Wisconsin Public Service Corporation
700 North Adams Street
Green Bay, Wisconsin 54301

Submitted by:
GEI Consultants, Inc.
3159 Voyager Drive
Green Bay, Wisconsin 54313
920-455-8200

October 2016, Revision 0

Project 1600630



John M. Trast, P.E.
Senior Consultant

Paul M. Garvey, P.G.
Senior Consultant

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1. Introduction

Wisconsin Public Service Corporation (WPSC) owns and operates the Weston Disposal Site No. 3 Landfill, located in the E 1/2 of the NW 1/4 and W 1/2 of the NE 1/4, Section 23, Township 26 North, Range 7 East, Town of Knowlton, Marathon County, Wisconsin. The WPSC Weston Disposal Site No. 3 Landfill is regulated as an industrial waste landfill by the Wisconsin Department of Natural Resources (WDNR) under the provisions of Chapter 289 Wisconsin State Statues, and all applicable requirements of Chapters NR 500 of the Wisconsin Administrative Code. The design, construction, operation, closure, and post-closure care requirements are specified in the WDNR conditionally approved Plan of Operations, License No. 3067, FID No. 737025120. The construction of Cells 1 and 2 commenced in May 2015. The landfill was placed into operation in 2016.

In addition to the state regulations, the landfill is also required to comply with 40 CFR Part 257 Subpart D – *Standards for Disposal of Coal Combustion Residuals in Landfills and Surface Impoundments.* Weston Disposal Site No. 3 Landfill, Cells 1 & 2 is defined as a CCR unit and existing CCR landfill in accordance with § 257.53 since construction commenced prior to October 14, 2015. Future landfill cells are permitted by the WDNR in the conditionally approved Plan of Operation and defined as lateral expansions under § 257.53 when constructed.

This report fulfills the requirements for a written Closure Plan of the Weston Disposal Site No. 3, Cells 1 and 2 in accordance with § 257.102 - *Criteria for Conducting the Closure or Retrofit of CCR Units*. In accordance with § 257.102(b)(1) this report describes the engineering design of the landfill, phased development, a description of the final cover system and how the final cover will be constructed, and how the final cover system will meet the applicable performance standards contained in § 257.102(d). In addition, it also includes an estimate of the maximum inventory of CCR, an estimate of the maximum open area that would require closure at one time, and a generalized schedule based on the anticipated landfill filling rates and disposal volumes.

This closure plan includes the following sections:

Section 1 Introduction

Section 2 Closure Narrative

Section 3 Final Cover System

Section 4 Schedule for Closure

Section 5 Conclusion and Certification

2. Closure Narrative

This section provides the closure narrative as required by § 257.102(b)(i). Closure of Cells 1 and 2 will be accomplished by leaving the CCR in place and installing a final cover meeting the requirements of § 257.102(d)(3) over the CCR. The final cover system is described in Section 3. The areal limits of Cells 1 and 2 are shown on drawing Sheet 6 of 29 – Cell 1 & 2 Site Preparation in Appendix A. Closure activities for Cell 1 and 2 will commence when CCR disposed in the cell reach final waste grades shown on drawings Sheet 7 of 29 – Cell 3 Site Preparation, Area A Closure and Sheet 14 of 29 – Top of Waste Grades in Appendix A. It will be necessary to laterally expand the landfill with the construction of Cell 3 before final waste grades are completed in Cell 1 and 2. At that time this closure plan will be updated to comply with the federal rules.

§ 257.102(b)(1)(iv) requires an estimate of the maximum inventory of CCR ever on the site over the active life of the CCR unit. The design capacity of Cell 1 and 2 is approximately 650,000 cubic yards. Therefore, prior to lateral expansion of the Weston Disposal Site No. 3 Landfill, in accordance with the approved Plan of Operation, the maximum CCR inventory of the landfill is 650,000 cubic yards.

§ 257.102(b)(1)(v) requires an estimate of the largest area of the CCR unit ever requiring final cover, at any time during the active life of the CCR unit. The area of Cells 1 and 2 is 15.2 acres. Therefore, prior to lateral expansion of the Weston Disposal Site No. 3 Landfill, in accordance with the approved Plan of Operation, the largest area of the CCR unit (Cell 1 and 2) requiring a final cover during the CCR unit's active life is 15.2 acres.

§ 257.102(d)(1)(i). The final cover system described in Section 3 is a composite final cover system which will envelop the CCR, minimizing post-closure infiltration and the potential release of CCR, leachate, or contaminated run-off from the closed unit. The landfill with the final cover is shown on Sheet 15 of 29 – Final Grades and final cover cross-sections are shown on drawings Sheet 19 of 29 – Engineering Cross-Section 324,700, Sheet 20 of 29 – Engineering Cross-Section 325,300, and Sheet 27 of 29 – Details. Fugitive dust from exposed CCR before and during final cover construction will be managed in accordance with the Fugitive Dust Control Plan. Surface water that has come into contact with CCR before and during final cover construction will be managed as leachate in accordance with the Run-on and Run-off Control Plan.

§ 257.102(d)(1)(iii). Slope stability of the CCR and final cover is enhanced in the manner in which the CCR is conditioned, placed, and compacted; how the facility is operated to promote storm and contact water management; and how the leachate collection system is designed and monitored to ensure leachate is being removed from the waste and not allowed to build-up within the landfill. The permitted final cover slopes will be at a 5% minimum slope at the top of the landfill to promote surface water drainage and prevent ponding due to the settlement of the final cover system. The perimeter side slopes of the landfill will be at a

maximum slope of 25% to provide long-term stable slopes that promote stormwater drainage, can be protected from excessive erosion, and safely maintained.

§ 257.102(d)(1)(iv). The final cover system described in Section 3 will minimize infiltration, which in turn minimizes the demand on the leachate collection system. The final cover will be vegetated with grass to promote evapotranspiration and prevent erosion. The final cover system vegetation will be maintained by fertilizing as necessary to develop a well-established vegetative cover and periodic mowing to stimulate root growth and prevent the establishment of woody vegetation. Final slopes will be between 5% and 25% to facilitate mowing. Slopes greater than 10% will be covered with erosion matting after seeding to minimize erosion during the establishment of vegetative cover.

§ 257.102(d)(1)(v). The final cover system described in Section 3 uses readily available equipment and materials and can easily be completed in a single construction season.

3. Final Cover System

This section is included to fulfill the requirements of § 257.102(b)(1)(iii).

Filling to final contours will result in a final slope no greater than 25% sloping downward from the center of the fill area to the perimeter of the site. The top portion of the landfill will be graded to no less than 5% sloping downward from the center to ensure positive drainage to the perimeter of the site. Drainage features, such as the perimeter ditches, terraces, and runoff channels will be constructed, as necessary, to accommodate surface runoff from phased closure.

The final cover system has been designed to minimize leachate generation by limiting percolation through the final cover barrier layer, promoting subsurface drainage to limit head on the barrier layer, and establishing vigorous plant growth to maximize evapotranspiration. The final cover system has also been designed for stability and to reduce maintenance. Specifically, the final cover from top down will consist of 6 inches of topsoil, 30 inches of general fill for the rooting zone layer, a geocomposite drainage layer, which may be substituted with select granular fill meeting NR 504 WAC, a 40-mil linear low density polyethylene geomembrane and either 24 inches of compacted clay, 24 inches of compacted ash, or a GCL with 24 inches of compacted barrier soils.

The hydraulic conductivity of the final cover system is required by § 257.102(d)(3)(i)(A) to be less than or equal to the hydraulic conductivity of the bottom liner system or natural subsoils present or a hydraulic conductivity no greater than 1.0E-05 cm/s, whichever is less. The Weston Disposal Site No. 3 Landfill is designed and constructed with a composite base liner system consisting of 2 feet of compacted soil, geosynthetic clay liner, and polyethylene geomembrane. The approved final cover system is a composite final cover consisting of a 2-foot compacted barrier layer, polyethylene geomembrane, drainage layer, and vegetated soil layers. The final cover system meets the requirements of § 257.102(d)(3)(i)(A).

Construction equipment and methods normally used in developing landfills and performing earth-moving projects will be used. The following sub-sections discuss the construction of the individual components of the final cover system. Layout and details of the final cover system are shown on the drawings included in Appendix A.

3.1 Compacted Barrier Layer

A minimum 2-foot-thick layer of compacted barrier layer constructed of clay, soil, or fly ash will be used as the soil component of the composite barrier layer. The materials will be placed and compacted with a large vibratory smooth-drum roller, with a minimum operating weight of 15,000 pounds, and while in vibratory mode, can provide 30,000 pounds of compactive energy. The barrier layer will be placed and compacted in lifts not exceeding 6 inches. The prepared barrier layer shall provide a firm, smooth surface for deployment of the geomembrane. The barrier layer should be free of any angular particles protruding from

the surface greater than 0.5 inches, sharp breaks in grade or excessive rutting greater than 0.2 feet. The completed barrier layer will have a maximum hydraulic conductivity of $5x10^{-5}$ cm/s. Based on the laboratory test results of the FGD filter cake and typical fly ash properties, the compaction and hydraulic conductivity specifications should be achievable using standard construction equipment and methods normally used in developing landfills and performing earth-moving projects.

3.2 Geomembrane

The geomembrane component of the final cover system will be a 40-mil textured linear low-density polyethylene (LLDPE) geomembrane. The LLDPE geomembrane has been selected in order to provide flexibility of the final cover system to accommodate expected settling and subsidence in accordance with § 257.102(d)(3)(i)(D). Geomembrane panels will be positioned by suspending rolls of material with a front-end loader and unrolling the suspended material by hand or with the aid of an ATV, as the loader remains stationary. The geomembrane will be installed in a loose and relaxed condition. Panels will be overlapped approximately 4 inches and fusion-welded together. At seam intersections and other repair locations, a geomembrane patch extending a minimum of 12 inches beyond the intersection or repair will be extrusion-welded into place. All seams will be non-destructively tested by air or vacuum testing. The integrity of fusion welds will be air tested, and extrusion welds will be vacuum-tested.

3.3 Drainage/Rooting Layer and Topsoil

A geocomposite drainage layer and a 30-inch-thick rooting zone layer meeting the requirements of § 257.102(d)(3)(i)(B) will be installed above the geomembrane final cover. The drainage layer will be installed to aid in the removal of subsurface storm water drainage; the rooting zone layer will be installed to support vegetative growth, and both layers will provide protection of the geomembrane and compacted barrier layer. The geocomposite will be deployed such that the seams run perpendicular to the contour lines of the slope to the extent possible. The geonet will be cable-tied every 3 feet along the edge of the panels and every 12 feet for end seams. The top geotextile will be sewn. The rooting layer will be placed over the geocomposite in a single lift using low ground pressure dozers. The material will be classified as SW, SP, SM, SC, ML, or CL and have a maximum particle size of 3 inches. The rooting layer will consist of on-site or off-site soils. As an alternative, the geocomposite drainage layer can be replaced by a 12-inch thick sand drainage layer having a minimum hydraulic conductivity of $1x10^{-3}$ cm/s and a maximum particle size of 0.25 inches. If the granular drainage layer is used the rooting zone thickness will be reduced to 18 inches.

Meeting the requirements of § 257.102(d)(3)(i)(C), topsoil capable of sustaining vegetative growth will be placed and spread into a uniform loose lift thickness of 6 inches. Once placed, the topsoil will be fertilized, seeded, and mulched. On all slopes greater than 10%, a temporary straw mulch blanket will be used to limit erosion and protect the seed prior to the establishment of vegetation.

4. Schedule for Closure

This section is included to fulfill § 257.102(b)(1)(v). The first phase of construction, Cells 1 and 2, were completed in 2015 and Cell 2 was placed into service in 2016. In accordance with the WDNR approved Plan of Operation, the landfill has a phased development plan, describing the construction, operation, and closure of each phase of the landfill from the construction of Cell 1 to the closure of Cell 9. In general, the development plan requires active landfill cells which have reached final waste grades be closed as soon as practical to limit the maximum open area, leachate generation, and the potential operational problems. A schedule for completing all activities necessary to satisfy the closure criteria is dependent on the CCR generation rates, beneficial reuse programs, and disposal rate volumes. However, final closure of Cells 1 and 2 will begin no later than 30 days following the final waste receipt for the CCR unit in accordance with §257.102(e)(1).

Final cover construction at the Weston Disposal Site No. 3 Landfill will be completed in accordance with the WDNR approved Plan of Operation under License No. 3067. Therefore no additional state or local approvals are required for WPSC to begin construction of the next phase of the landfill or closure of an existing phase. The final cover system described in Section 3 uses standard and readily available equipment and materials and can easily be completed in a single construction season.

5. Conclusion and Certification

WPSC owns and operates the Weston Disposal Site No. 3 Landfill, located in the E 1/2 of the NW 1/4 and W 1/2 of the NE 1/4, Section 23, Township 26 North, Range 7 East, Town of Knowlton, Marathon County, Wisconsin. The Weston Disposal Site No. 3 Landfill is required to comply with 40 CFR Part 257 Subpart D – *Standards for Disposal of Coal Combustion Residuals in Landfills and Surface Impoundments*. This plan fulfills the requirements for a written Closure Plan of the Weston Disposal Site No. 3 Landfill , Cell 1 and 2 in accordance with § 257.102 - *Criteria for Conducting the Closure or Retrofit of CCR Units*, describing the engineering design and construction of the final cover system, how the final cover system will meet the applicable performance standards contained in § 257.102(d), an estimate of the maximum inventory of CCR, an estimate of the maximum open area that would require closure at one time, and a generalized schedule based on the anticipated landfill filling rates and disposal volumes.

The Closure Plan was completed under the direction of John M. Trast, P.E. I am a licensed professional engineer in the State of Wisconsin in accordance with the requirements of ch. A-E 4, Wisconsin Administrative Code; that this document has been prepared in accordance with the Rules of Professional Conduct in ch. A-E 8, Wisconsin Administrative Code; and that, to the best of my knowledge, all information contained in this document is correct and the document was prepared in compliance with all applicable requirements in 40 CFR Part 257 Subpart D.

Appendix A

Weston Disposal Site No. 3, WDNR License No. 3067, Wisconsin Public Service Corporation, Plan of Operation Drawings

Sheet 6 of 29 – Cell 1 & 2 Site Preparation
Sheet 7 of 29 – Cell 3 Site Preparation, Area A Closure
Sheet 14 of 29 – Top of Waste Grades
Sheet 15 of 29 – Final Grades
Sheet 19 of 29 – Engineering Cross-Section 324,700
Sheet 20 of 29 – Engineering Cross-Section 325,300
Sheet 27 of 29 – Details.

Cell 1 Pipe Lengths (Leachate and Gradient Control)	
3-in.(SDR 17) HDPE/(SCH 120) PVC leachate headwell pipe	210 LF
3-in. inside 6-in-dia. /(SDR 17) HDPE (SCH 80) PVC forcemain	2,870 LF
8-in. (SDR 11) HDPE/ (SCH 80) PVC leachate col. pipe (perf.)	1,440 LF
8-in. (SDR 11) HDPE/(SCH 80) PVC leachate col. pipe (non-perf.)	1,490 LF
6-in. (SDR 11) HDPE/(SCH 80) PVC gradient pipe (perf.)	1,440 LF
6-in. (SDR 11) HDPE/(SCH 80) PVC gradient pipe (non-perf.)	350 LF
18-in. (SDR 17) HDPE/(SCH 80) PVC riser pipe (perf.)	30 LF
18-in. (SDR 17) HDPE/(SCH 80) PVC riser pipe (non-perf.)	80 LF

Cell 2 Pipe Lengths (Leachate and Gradient Control)	
3-in.(SDR 17) HDPE/(SCH 120) PVC leachate headwell pipe	270 LF
3-in. inside 6-in-dia. /(SDR 17) HDPE (SCH 80) PVC forcemain	2,010 LF
8-in. (SDR 11) HDPE/ (SCH 80) PVC leachate col. pipe (perf.)	1,420 LF
8-in. (SDR 11) HDPE/(SCH 80) PVC leachate col. pipe (non-perf.)	140 LF
6-in. (SDR 11) HDPE/(SCH 80) PVC gradient pipe (perf.)	1,420 LF
6-in. (SDR 11) HDPE/(SCH 80) PVC gradient pipe (non-perf.)	350 LF
18-in. (SDR 17) HDPE/(SCH 80) PVC riser pipe (perf.)	30 LF
18-in. (SDR 17) HDPE/(SCH 80) PVC riser pipe (non-perf.)	140 LF

Cell 1 Miscellaneous Items	Quantities
Perimeter access manhole	2 EA
Submersible pump with control panel	2 EA
Abandon monitoring well(s)	2 EA
Culvert(s)	0 EA
Storm water basin	2 EA
Drainage ditches	0 EA
Riprap apron(s)	0 EA
Leachate transfer manholes	10 EA
Ceanouts	7 EA
Surface water drop manholes	2 EA

Cell 2 Miscellaneous Items	Quantities
Perimeter access manhole	2 EA
Submersible pump with control panel	2 EA
Abandon monitoring well(s)	2 EA
Culvert(s)	0 EA
Dewater and excavate existing sedimentation basin	1 EA
Storm water basin	2 EA
Drainage ditches	0 EA
Riprap apron(s)	0 EA
Leachate transfer manholes	5 EA
Ceanouts	2 EA
Surface water drop manholes	1 EA

NOTES

1. REFER TO PLAN SHEET 2 OF THIS PLAN SET FOR BASE MAP NOTES AND LEGEND.
2. AUXILIARY SEDIMENT TRAPS WILL BE CONSTRUCTED AS NECESSARY DURING DEVELOPMENT OF EACH CELL.

Cell 1 and 2 Liner Construction Activities

Site Preparation (construction sequence may vary)

1 Locate utilities and relocate as required.
2 Locate and mark wetland boundaries.
3 Abandon LS-9OW, LS-9P, LS-22R, LS-22P,LS-56 OW,LS-56P, & LS-57.
4 Demolish or relocate existing structures including fence within Cell 1.
5 Install erosion control structures.
6 Clear and grub entire site.
7 Strip topsoil in areas of Cell 1, Cell 2, stockpile, and storm water basins.
8 Install temporary haul/access roads.
9 Dewater and excavate existing sedimentation basin.
10 Relocate existing waste within limits of existing landfill for Cell 1 const.
11 Construct subbase grades, perimeter berms, and sedimentation basins.
12 Install surface water drop manholes
13 Excavate GCS collection trenches, install select granular fill, geotextile, pipes, aggregate fill, undercut leachate collection lines & sumps.
14 Construct the compacted clay liner.
15 Install geosynthetic clay liner (GCL) and HDPE geomembrane in Cell 1.
16 Install sumps, cleanout riser pipes, collection lines, and cleanouts in Cell 1.
17 Install leachate headwells in Cell 1.
18 Install cell delineation berm between Cell 1 and Cell 2.
19 Install leachate collection tank, manhole(s), and forcemain piping.
20 Install 1' thick select granular fill drainage layer in Cell 1.
21 Excavate & relocate existing waste into Cell 1 (after Cell 1 doc rpt is app'd).
22 Excavate and stockpile in-place clay barrier layer for re-use.
23 Remove leachate collection tank and LS-57.
24 Complete construction of subbase grades in Cell 2 area.
25 Complete construction of the compacted clay liner for Cell 2.
26 Install geosynthetic clay liner (GCL) and HDPE geomembrane in Cell 2.
27 Install leachate sumps, cleanout riser pipes, collection lines, &cleanouts.
28 Install leachate headwells in Cell 2.
29 Install cell delineation berm between Cell 2 and Cell 3.
30 Install 1-foot thick select granular fill drainage layer in Cell 2.
31 Install perimeter access road.

Cell 1 Construction Materials	Volume/Area
Topsoil stockpile volume	9,700
Fill to construct perimeter berms and storm water basin	104,300
Geotextile for gradient control trench (in-place quantity)	1,400
Select aggregate fill for gradient control trench	210
Select granular fill for gradient control system	2,760
Compacted select clay fill (in-place volume)	21,300
Cell delineation berm	700
GCL	31,900
60 mil geomembrane liner (in-place quantity)	31,900
Geotextile for cushion under pipe bedding (in-place quantity)	4,900
Select aggregate fill for leachate collection trenches and sumps	1,800
Select granular fill for leachate drainage blanket	10,600
Perimeter access road	1,200

Cell 2 Construction Materials	Volume/Area
Topsoil stockpile volume	9,700
Fill to construct perimeter berms and storm water basin	148,400
Geotextile for gradient control trench (in-place quantity)	1,400
Select aggregate fill for gradient control trench	210
Select granular fill for gradient control drainage blanket	2,670
Compacted select clay fill (in-place volume)	27,400
Cell delineation berm	750
GCL	41,100
60 mil geomembrane liner (in-place quantity)	41,100
Geotextile for cushion under pipe bedding (in-place quantity)	2,600
Select aggregate fill for leachate collection trenches and sumps	950
Select granular fill for leachate drainage blanket	13,700
Perimeter access road	1,250

0 100 200 300 400
SCALE IN FEET

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NO.	BY	DATE	REVISION	APP'D.
3.				
2.				
1.				

PROJECT: WISCONSIN PUBLIC SERVICE CORPORATION
WESTON DISPOSAL SITE NO. 3 EXPANSION
PLAN OF OPERATION

SHEET TITLE:
CELL 1 & CELL 2 SITE PREPARATION

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CHECKED BY:	TDH	1"=100'	FILE NO. 196089.0003.SHT06-PH.dwg
APPROVED BY:	CDM	DATE PRINTED:	SHEET 6 OF 29
DATE:	MARCH 2014		

TRC
708 Heartland Trail
Suite 3000
Madison, WI 53717
Phone: 608.826.3600

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NOTES

1. REFER TO PLAN SHEET 2 OF THIS PLAN SET FOR BASE MAP NOTES AND LEGEND.
2. AUXILIARY SEDIMENT TRAPS WILL BE CONSTRUCTED AS NECESSARY DURING DEVELOPMENT OF EACH CELL.
3. TOPSOIL STRIPPING/STOCKPILING FOR CELL 3 WAS COMPLETED DURING CELL 1/CELL 2 CONSTRUCTION.
4. THE ENTIRE SITE WAS CLEARED AND GRUBBED DURING CELL 1/CELL 2 CONSTRUCTION.

Cell 3 Liner Construction Activities

Cell 3 Site Preparation (construction sequence may vary)

1 Locate utilities and relocate as required.
2 Locate and mark wetland boundaries.
3 Install erosion control structures.
4 Clear and grub.
5 Strip topsoil and stockpile.
6 Install temporary haul/access roads.
7 Construct subbase grades and perimeter berms.
8 Excavate GCS trenches, install select granular fill, geotextile, pipes, aggregate fill, and undercut leachate col. lines/sumps.
9 Construct the compacted clay liner.
10 Install geosynthetic clay liner (GCL) and HDPE geomembrane.
11 Install leachate sumps, riser pipes, collection lines, and cleanouts.
12 Install leachate headwells.
13 Install cell delineation berms.
14 Install leachate transfer manhole(s) and forcemain piping.
15 Install 1-foot thick select granular fill drainage layer.
16 Install perimeter access road.
17 Seed, fertilize, and mulch construction areas at finish grade.

Area A Final Cover Construction Activities

Site Preparation (Construction sequence may vary)

1 Install erosion control devices
2 Grade waste to proposed top of waste (base of final cover) elevations.
3 Construct the 2 foot thick clay, soil, or fly ash barrier layer.
4 Install GCL layer (if fly ash or soil barrier layer is utilized).
5 Install 40 mil geomembrane layer.
6 Install final cover drainage layer discharge piping.
7 Install geonet (geotextile/geonet/geotextile) or sand drainage layer.
8 Construct general fill rooting zone layer.
9 Install downslope flume pipe(s) and energy dissipater(s).
10 Install diversion berms.
11 Construct topsoil layer.
12 Seed, fertilize, and mulch construction areas at finish grade.

Cell 3 Construction Materials	Volume/Area
Topsoil stockpile volume	10,700 CY
Fill to construct perimeter berms and storm water basin	29,800 CY
Geotextile for gradient control trench (in-place quantity)	750 SY
Select aggregate fill for gradient control trench	240 CY
Select granular fill for gradient control drainage blanket	1,360 CY
Compacted select clay fill (in-place volume)	13,600 CY
Cell delineation berm	750 LF
GCL	20,300 SY
60 mil geomembrane liner (in-place quantity)	20,300 SY
Geotextile for cushion under pipe bedding (in-place quantity)	1,300 SY
Select aggregate fill for leachate collection trenches and sumps	490 CY
Select granular fill for leachate drainage blanket	6,800 CY
Perimeter access road	225 LF
Cell 3 Pipe Lengths (Leachate and Gradient Control)	
3-in.(SDR 17) HDPE/(SCH 120) PVC leachate headwell pipe	280 LF
3-in. inside 6-in-dia. /(SDR 17) HDPE (SCH 80) PVC forcemain	0 LF
6-in. (SDR 11) HDPE/ (SCH 80) PVC leachate col. pipe (perf.)	725 LF
8-in. (SDR 11) HDPE/(SCH 80) PVC leachate col. pipe (non-perf.)	70 LF
6-in. (SDR 11) HDPE/(SCH 80) PVC gradient pipe (perf.)	725 LF
6-in. (SDR 11) HDPE/(SCH 80) PVC gradient pipe (non-perf.)	210 LF
18-in. (SDR 17) HDPE/(SCH 80) PVC riser pipe (perf.)	15 LF
18-in. (SDR 17) HDPE/(SCH 80) PVC riser pipe (non-perf.)	70 LF

Miscellaneous Items	Quantities
Perimeter access manhole	1 EA
Submersible pump with control panel	1 EA
Abandon monitoring well(s)	0 EA
Culvert(s)	0 EA
Storm water basin	0 EA
Drainage ditches	0 EA
Riprap apron(s)	11 EA
Leachate Transfer Manholes	0 EA
Ceanouts	1 EA
Surface water drop manholes	0 EA

Area A Construction Materials	Quantities
Topsoil (in-place volume)	7,900 CY
General fill (2.5--thick rooting zone layer, in-place volume)	39,500 CY
Select clay, soil, or fly ash barrier layer (in-place volume)	31,600 CY
GCL	47,400 SY
40 mil geomembrane liner (in-place quantity)	47,400 SY
Geonet drainage layer (in-place quantity)	47,400 SY
Sand drainage layer (if utilized)	15,800 CY
4-in--dia. (SDR 17), (ADS) or (SCH 80) toe drain discharge pipe	2,510 LF
Select aggregate fill for toe drain discharge pipe trench	400 LF
HDPE downslope flume pipe (elbows, aprons, connections)	3 EA
Energy dissipater	0 EA
Surface water diversion berm	5,000 LF

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NO.	BY	DATE	REVISION	APPD.
3.				
2.				
1.				

PROJECT: WISCONSIN PUBLIC SERVICE CORPORATION
WESTON DISPOSAL SITE NO. 3 EXPANSION
PLAN OF OPERATION

SHEET TITLE: CELL 3 SITE PREPARATION, AREA A CLOSURE

DRAWN BY:	LSTORMER	SCALE:	1"=100'
CHECKED BY:	TDH	PROJ. NO.	196089.0003
APPROVED BY:	CDM	FILE NO.	196089.0003.SHT07-PH.dwg
DATE:	MARCH 2014	DATE PRINTED:	

SHEET 7 OF 29

TRC
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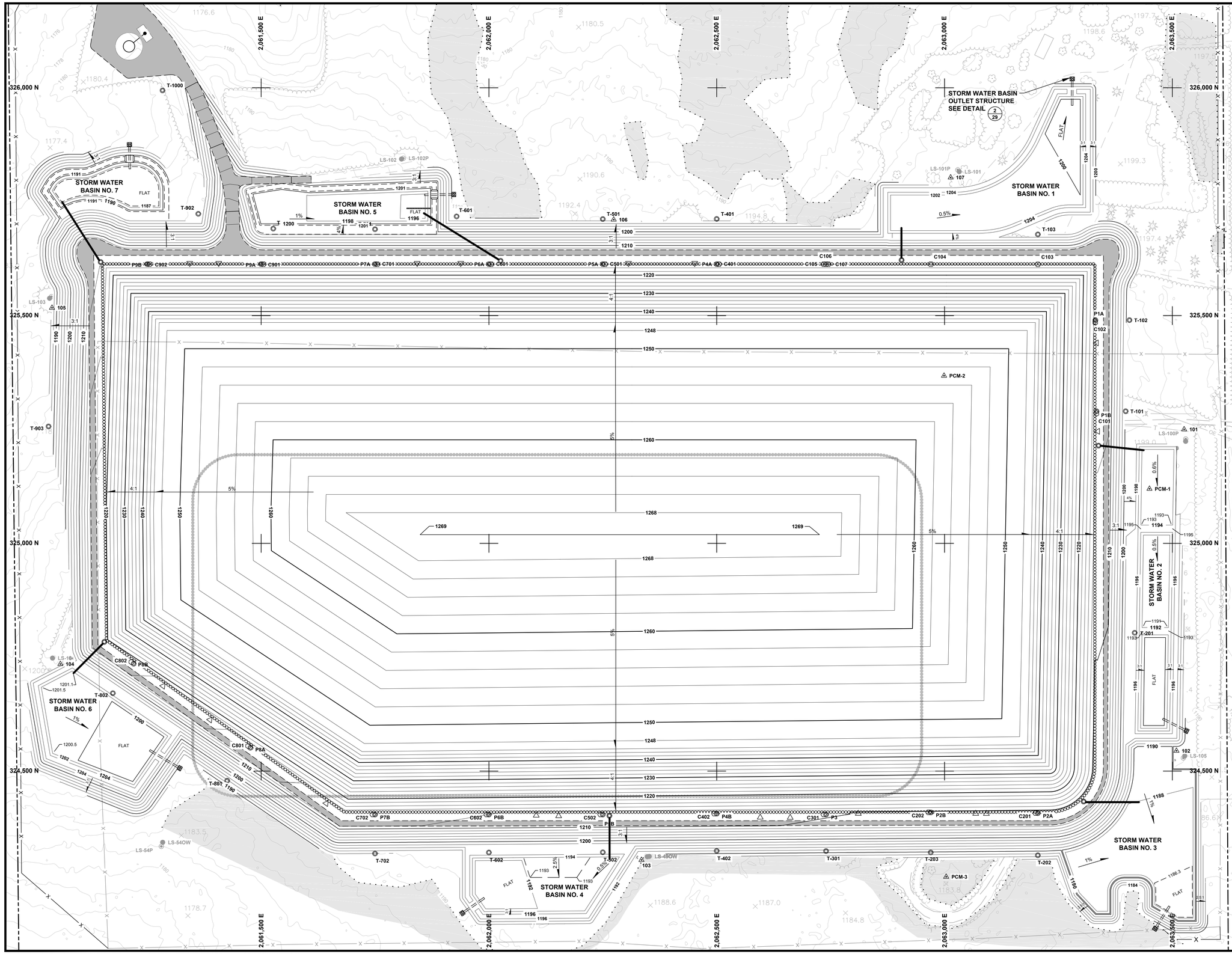
Phone: 608.826.3600



NOTES:

1. REFER TO PLAN SHEET 2 FOR BASE MAP NOTES.
2. CONTOURS REPRESENT TOP OF WASTE (BOTTOM OF FINAL COVER) WITHIN THE LIMITS OF WASTE AND FINISHED GRADE (TOP OF TOPSOIL, ETC.) BEYOND (EXCEPT AS NOTED ON DETAILS).
3. DAILY COVER WILL NOT BE UTILIZED.
4. INTERMEDIATE COVER MAY BE UTILIZED FOR TEMPORARY SLOPES DEPENDING ON WASTE GENERATION RATES, BENEFICIAL REUSE AND THE FINAL COVER CONSTRUCTION SCHEDULE.

0 100 200 300 400
SCALE IN FEET

NOTE: THESE PLANS ARE ACCOMPANIED BY A REPORT OF THE SAME TITLE. THESE DOCUMENTS ARE INTERRELATED AND INTENDED TO BE USED AND REVIEWED TOGETHER. THESE DOCUMENTS ARE INTENDED TO BE USED FOR REGULATORY PURPOSES ONLY.

NOT FOR CONSTRUCTION

NO.	BY	DATE	REVISION	APPD.
3.				
2.				
1.				

PROJECT: **WISCONSIN PUBLIC SERVICE CORPORATION**
WESTON DISPOSAL SITE NO. 3 EXPANSION
PLAN OF OPERATION

SHEET TITLE:

TOP OF WASTE GRADES

DRAWN BY:	LSTORMER	SCALE:	PROJ. NO. 196089.0003
CHECKED BY:	TDH	1"=100'	FILE NO. 196089.0003.SHT14-TW.dwg
APPROVED BY:	CDM	DATE PRINTED:	**SHEET 14 OF 28**
DATE:	MARCH 2014		

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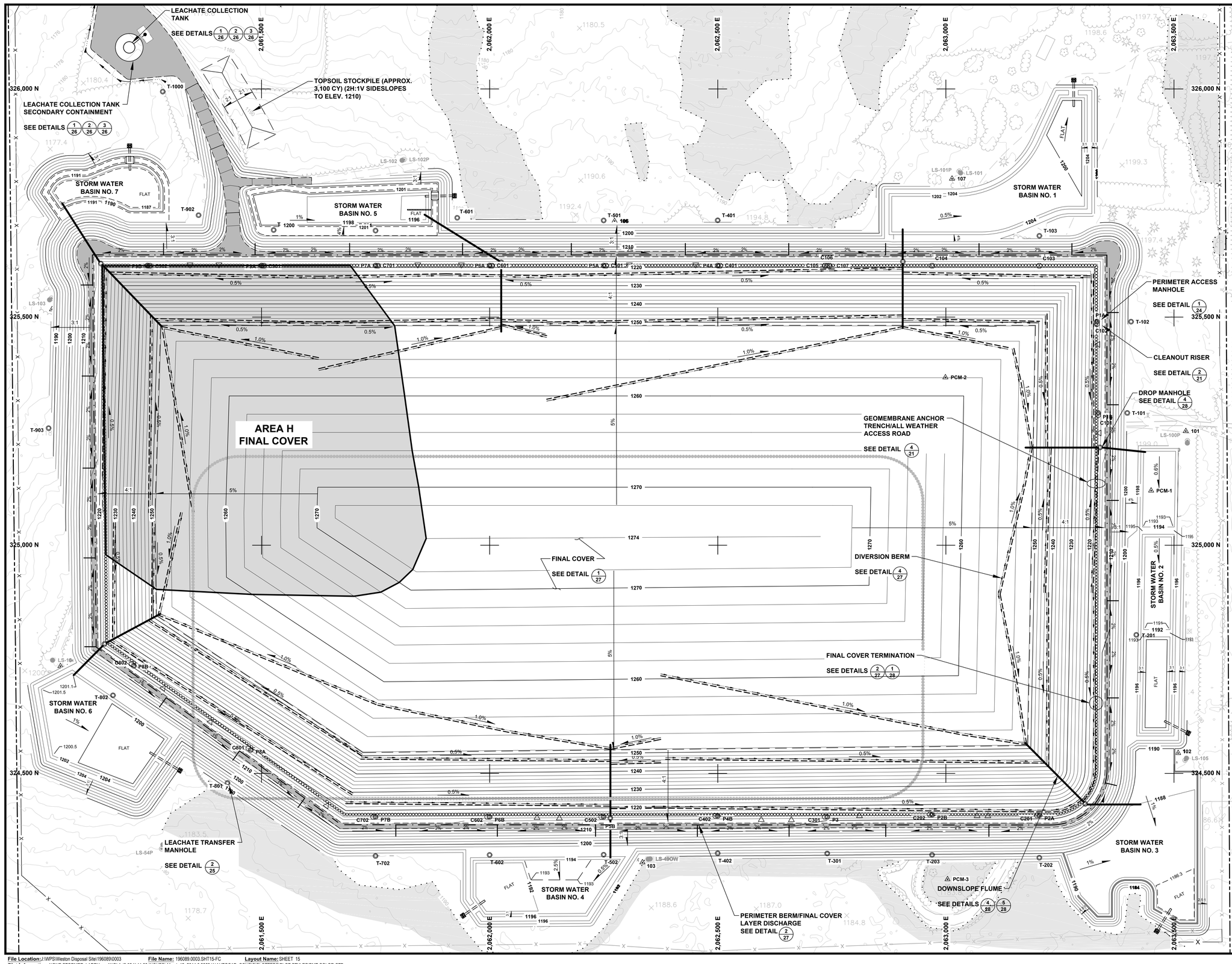
708 Heartland Trail
Suite 3000
Madison, WI 53717
Phone: 608.826.3600

File Location: J:\WPS\Weston Disposal Site\196089\0003 **File Name:** 196089.0003.SHT14-TW **Layout Name:** SHEET 14
Plot Information: NONE STORMER, LARRY — ANSI A (8.50 X 11.00 INCHES) March 13, 2014 0.3869 K:\AUTOCAD_CONFIG\PLOTTERS\PLOT STYLES\RMT COLOR.CTB



NOTES

1. REFER TO PLAN SHEET 2 FOR STANDARD LEGEND AND BASE MAP NOTES.
2. CONTOURS REPRESENT FINAL GRADES (TOP OF FINAL COVER) WITHIN THE LIMITS OF WASTE AND FINISHED GRADE (TOP OF TOPSOIL, TOP OF ROAD SURFACE, ETC.) BEYOND.
3. ALL CLEARING AND GRUBBING WILL OCCUR DURING CELL 1/CELL 2 CONSTRUCTION.
4. LOCATIONS OF FINAL COVER DRAINAGE LAYER DISCHARGE PIPE MAY VARY BASED UPON WASTE DISPOSAL RATES AND AREAS OF FINAL COVER CONSTRUCTION.

Area H Final Cover Construction Activities

Site Preparation

1 Install erosion control devices.
2 Grade waste to proposed top of waste (base of final cover) elevations.
3 Construct the 2 foot thick clay, soil, or fly ash barrier layer and GCL layer.
4 Install GCL layer (if fly ash or soil barrier layer is utilized).
5 Install 40 mil geomembrane layer.
6 Install final cover drainage layer discharge piping.
7 Install geonet (geotextile/geonet/geotextile) or sand drainage layer.
8 Construct general fill rooting zone layer.
9 Install downslope flume pipe(s) and energy dissipater(s).
10 Install diversion berms.
11 Construct topsoil layer.
12 Seed, fertilize, and mulch construction areas at finish grade.

Area H Construction Materials	Quantities
Topsoil (in-place volume)	8,400 CY
General fill (2.5'-thick rooting zone layer, in-place volume)	42,000 CY
Select clay, soil, or fly ash barrier layer (in-place volume)	33,600 CY
GCL	50,300 SY
40 mil geomembrane liner (in-place quantity)	50,300 SY
Geonet drainage layer (in-place quantity)	50,300 SY
Sand drainage layer (if utilized)	16,800 CY
4-in-dia. (SDR 17), (ADS) or (SCH 80) toe drain discharge pipe	1,420 LF
Select aggregate fill for toe drain discharge pipe trench	240 CY
HDPE downslope flume pipe (elbows, aprons, connections)	1 EA
Energy dissipater	0 EA
Surface water diversion berm	2,900 LF

Miscellaneous Items	Quantities
Culvert(s)	0 EA
Storm water basin	1 EA
Drainage ditches	0 EA
Riprap apron(s)	6 EA

0 100 200 300 400
SCALE IN FEET

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NO.	BY	DATE	REVISION	APPD.
3.				
2.				
1.				

PROJECT: **WISCONSIN PUBLIC SERVICE CORPORATION**
WESTON DISPOSAL SITE NO. 3 EXPANSION
PLAN OF OPERATION

SHEET TITLE: **FINAL GRADES**

DRAWN BY:	LSTORMER	SCALE:	PROJ. NO. 196089.0003
CHECKED BY:	TDH	1"=100'	FILE NO. 196089.0003.SHT15-FC.dwg
APPROVED BY:	CDM	DATE PRINTED:	SHEET 15 OF 29
DATE:	MARCH 2014		

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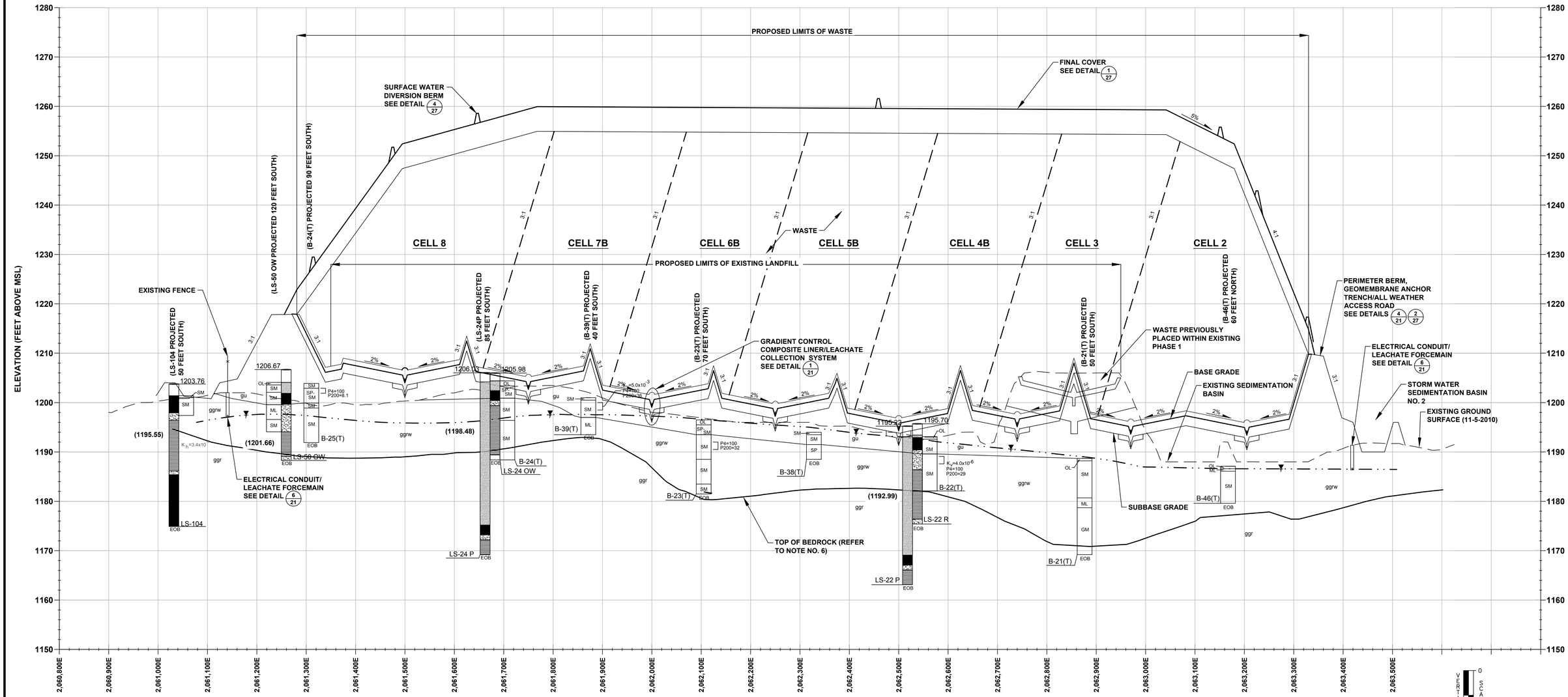
File Location: J:\WPS\Weston Disposal Site\196089\0003 **File Name:** 196089.0003.SHT15-FC **Layout Name:** SHEET 15
Plot Information: NONE STORMER, LARRY — ANSI A (8.50 X 11.00 INCHES) March 13, 2014 0.3869 K:\AUTOCAD_CONFIG\PLOTTERS\PLOT STYLES\RMT COLOR.CTB



CROSS SECTION 324,700N (LOOKING NORTH)

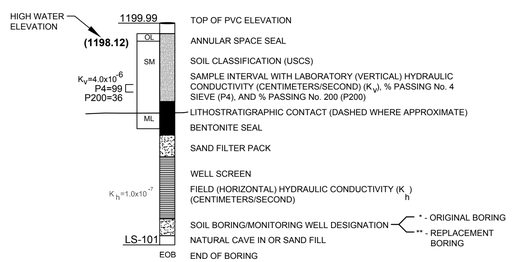
VERTICAL SCALE 0 10

HORIZONTAL SCALE 0 100 200

VERTICAL EXAGGERATION = 10X

LEGEND

- EXISTING GROUND SURFACE
- LITHOSTRATIGRAPHIC UNIT (DASHED WHERE APPROXIMATE)
- COMPETENT BEDROCK SURFACE (DASHED WHERE APPROXIMATE)
- HIGH TABLE SURFACE - (REFER TO NOTE NO. 5 BELOW)



STRATIGRAPHIC UNIT

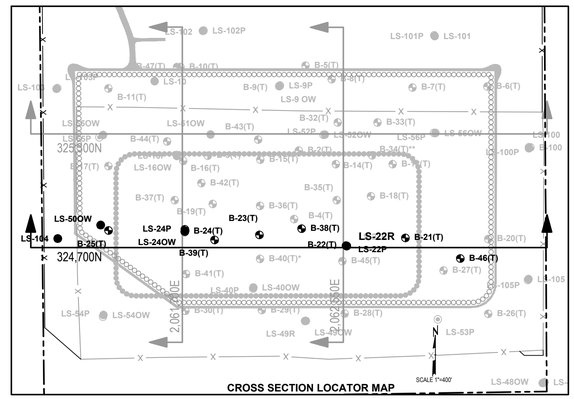
PLEISTOCENE	gu -	MARATHON FORMATION, UNDIFFERENTIATED (Attig and Muldoon, 1989) BROWN TO DARK BROWN SAND TO SILTY SAND DERIVED FROM TILL, RESIDUUM, OR HILLSLOPE SEDIMENT
	ggrw -	WEATHERED BEDROCK. YELLOWISH BROWN TO DARK BROWN SILTY SAND DERIVED FROM LOWER PROTEROZOIC BEDROCK (ggr).
LOWER PROTEROZOIC	ggr -	GNEISSIC GRANITE (LaBerge and Myers, 1983) LIGHT GREY TO PINK QUARTZ DIORITE AND RED TO PINK GRANITE, WITH TRACE BLACK AMPHIBOLITE

UNIFIED SOIL CLASSIFICATION

GW	WELL GRADED GRAVELS & GRAVEL WITH SAND
GP	POORLY GRADED GRAVELS & GRAVEL WITH SAND
GM	SILTY GRAVELS, SILTY GRAVELS WITH SAND
GC	CLAYEY GRAVELS, CLAYEY GRAVEL WITH SAND
SW	WELL GRADED SANDS & SAND WITH GRAVEL
SP	POORLY GRADED SANDS & SANDS WITH GRAVEL
SP-SM	POORLY GRADED SAND WITH SILT, SAND WITH SILT & GRAVEL
SM	SILTY SANDS, SILTY SAND WITH GRAVEL
SC	CLAYEY SANDS, CLAYEY SANDS WITH GRAVEL
CH	HIGH PLASTICITY CLAY
CL-ML	SILTY CLAY TO CLAYEY SILT
CL	LOW PLASTICITY CLAY, GRAVELLY-SANDY CLAYS
ML	SILT, GRAVELLY-SANDY SILT
OL	ORGANIC CLAY, SANDY-GRAVELLY ORGANIC SOIL
OH	ORGANIC, HIGH PLASTICITY, SILTY CLAY
PT	PEAT, MUCK, ORGANIC SOILS

NOTES

1. THE CORRELATION LINES ARE BASED ON INTERPOLATION BETWEEN BORINGS AND MAY NOT REPRESENT ACTUAL SUBSURFACE CONDITIONS.
2. HORIZONTAL DISTANCES ARE MEASURED AND WATER TABLE DATA ARE PLOTTED WITH RESPECT TO THE CENTER OF EACH SOIL BORING LOCATION. WELL CONSTRUCTION SCHEMATICS ARE OFFSET FOR CLARITY.
3. THE EXISTING GROUND SURFACE/SUBBASE GRADES/BASEGRADES/FINAL GRADES ARE BASED ON THE INFORMATION PRESENTED ON PLAN SHEETS 3, 4, 5, AND 15 RESPECTIVELY.
4. ELEVATIONS ARE SHOWN IN REFERENCE TO USGS MEAN SEA LEVEL (MSL) DATUM.
5. HIGH WATER TABLE BASED ON SURFACE PRESENTED ON HIGH WATER TABLE MAP FR-6 (REV. 1) THAT WAS SUBMITTED WITHIN ADDENDUM NO. 2 OF THE FEASIBILITY REPORT, DATED JULY 18, 2013. THE WATER TABLE SURFACE IS BASED UPON DEPTH TO WATER MEASUREMENTS RECORDED ON APRIL 22, 2013 (FOR EXISTING MONITORING WELLS INSTALLED PRIOR TO 2011) AND WATER DEPTH TO WATER MEASUREMENTS RECORDED ON MAY 13, 2013 (FOR THE 100 - SERIES MONITORING WELLS).
6. TOP OF BEDROCK SURFACE IS BASED ON A DIGITAL TERRAIN MODEL (DTM) PREPARED WITH THE TOP OF BEDROCK ELEVATIONS AT EACH SOIL BORING LOCATION.
7. FOR DETAILED LITHOLOGICAL DESCRIPTIONS, REFER TO WELL CONSTRUCTION DETAILS AND THE SOILS LABORATORY TEST RESULTS IN THE FEASIBILITY REPORT.
8. THE DEVELOPMENT/PROGRESSION OF EACH CELL MAY VARY BASED UPON WASTE DISPOSAL SCHEDULE, BENEFICIAL REUSE PROJECT OPPORTUNITIES, AND THE NUMBER/SIZE OF BULK-HAULING EVENTS FROM EACH OF THE POWER PLANTS.



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NOT FOR CONSTRUCTION

NO.	BY	DATE	REVISION	APP'D.
3.				
2.				
1.				

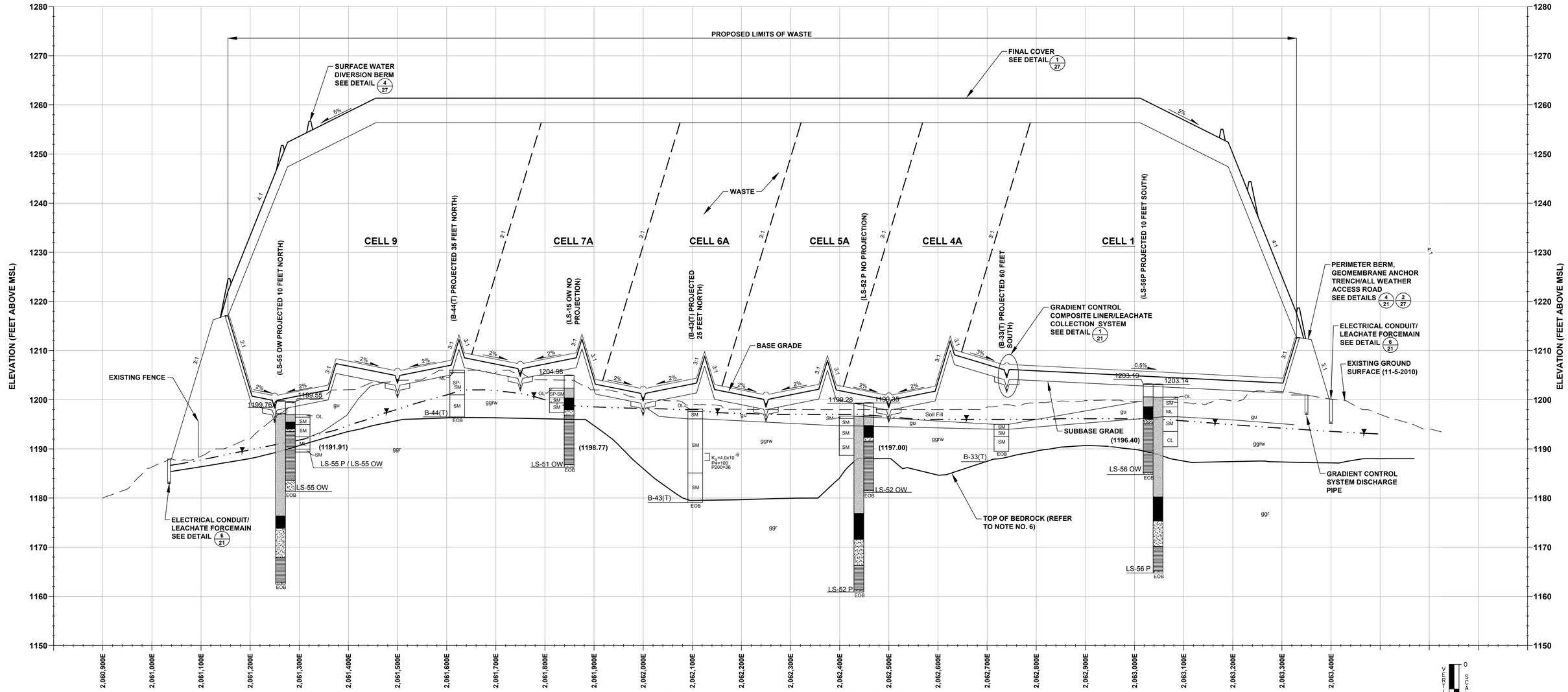
PROJECT: **WISCONSIN PUBLIC SERVICE CORPORATION**
WESTON DISPOSAL SITE NO. 3 EXPANSION
PLAN OF OPERATION

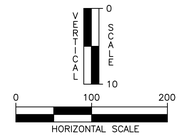
SHEET TITLE:
ENGINEERING CROSS SECTION 324,700N

DRAWN BY:	LSTORMER	SCALE:	PROJ. NO. 196089.0003
CHECKED BY:	TDH	AS SHOWN	FILE NO. 196089.0003.SHT17-20-XS.dwg
APPROVED BY:	CDM	DATE PRINTED:	**SHEET 19 OF 29**
DATE:	MARCH 2014		

TRC
708 Heartland Trail
Suite 3000
Madison, WI 53717
Phone: 608.826.3600

File Location: J:\WPS\Weston Disposal Site\196089\0003 **File Name:** 196089.0003.SHT17-20-XS **Layout Name:** SHEET 19
Plot Information: NONE STORMER, LARRY — ANSI A (8.50 X 11.00 INCHES) March 13, 2014 0.3869 K:\AUTOCAD_CONFIG\PLOTTERS\PLOT STYLES\RMT COLOR.CTB





VERTICAL EXAGGERATION = 10X

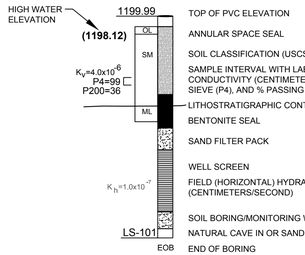
CROSS SECTION 325,300N (LOOKING NORTH)

NOTES

1. THE CORRELATION LINES ARE BASED ON INTERPOLATION BETWEEN BORINGS AND MAY NOT REPRESENT ACTUAL SUBSURFACE CONDITIONS.
2. HORIZONTAL DISTANCES ARE MEASURED AND WATER TABLE DATA ARE PLOTTED WITH RESPECT TO THE CENTER OF EACH SOIL BORING LOCATION. WELL CONSTRUCTION SCHEMATICS ARE OFFSET FOR CLARITY.
3. THE EXISTING GROUND SURFACE/SUBBASE GRADES/BASEGRADES/FINAL GRADES ARE BASED ON THE INFORMATION PRESENTED ON PLAN SHEETS 3, 4, 5, AND 15 RESPECTIVELY.
4. ELEVATIONS ARE SHOWN IN REFERENCE TO USGS MEAN SEA LEVEL (MSL) DATUM.
5. HIGH WATER TABLE BASED ON SURFACE PRESENTED ON HIGH WATER TABLE MAP FR-6 (REV. 1) THAT WAS SUBMITTED WITHIN ADDENDUM NO. 2 OF THE FEASIBILITY REPORT, DATED JULY 18, 2013. THE WATER TABLE SURFACE IS BASED UPON DEPTH TO WATER MEASUREMENTS RECORDED ON APRIL 22, 2013 (FOR EXISTING MONITORING WELLS INSTALLED PRIOR TO 2011) AND WATER DEPTH TO WATER MEASUREMENTS RECORDED ON MAY 13, 2013 (FOR THE 100 - SERIES MONITORING WELLS).
6. TOP OF BEDROCK SURFACE IS BASED ON A DIGITAL TERRAIN MODEL (DTM) PREPARED WITH THE TOP OF BEDROCK ELEVATIONS AT EACH SOIL BORING LOCATION.
7. FOR DETAILED LITHOLOGICAL DESCRIPTIONS, REFER TO WELL CONSTRUCTION DETAILS AND THE SOILS LABORATORY TEST RESULTS IN THE FEASIBILITY REPORT.
8. THE DEVELOPMENT/PROGRESSION OF EACH CELL MAY VARY BASED UPON WASTE DISPOSAL SCHEDULE, BENEFICIAL REUSE PROJECT OPPORTUNITIES, AND THE NUMBER/SIZE OF BULK-HAULING EVENTS FROM EACH OF THE POWER PLANTS.

LEGEND

- EXISTING GROUND SURFACE
- LITHOSTRATIGRAPHIC UNIT (DASHED WHERE APPROXIMATE)
- COMPETENT BEDROCK SURFACE (DASHED WHERE APPROXIMATE)
- HIGH TABLE SURFACE - (REFER TO NOTE NO. 5 BELOW)

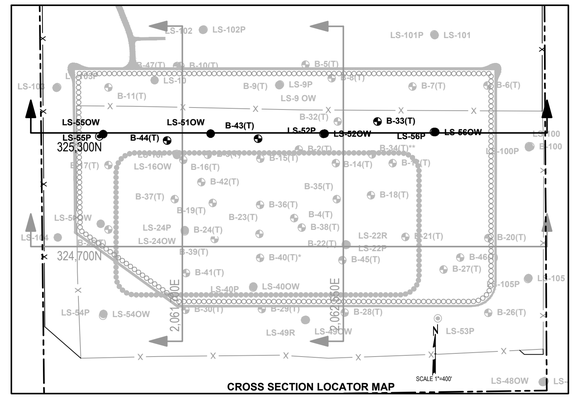


STRATIGRAPHIC UNIT

PLEISTOCENE	gu -	MARATHON FORMATION, UNDIFFERENTIATED (Attig and Muldoon, 1989) BROWN TO DARK BROWN SAND TO SILTY SAND DERIVED FROM TILL, RESIDUUM, OR HILLSLOPE SEDIMENT
	ggrw -	WEATHERED BEDROCK. YELLOWISH BROWN TO DARK BROWN SILTY SAND DERIVED FROM LOWER PROTEROZOIC BEDROCK (ggr).
LOWER PROTEROZOIC	ggr -	GNEISSIC GRANITE (LaBerge and Myers, 1983) LIGHT GREY TO PINK QUARTZ DIORITE AND RED TO PINK GRANITE, WITH TRACE BLACK AMPHIBOLITE

UNIFIED SOIL CLASSIFICATION

GW	WELL GRADED GRAVELS & GRAVEL WITH SAND
GP	POORLY GRADED GRAVELS & GRAVEL WITH SAND
GM	SILTY GRAVELS, SILTY GRAVELS WITH SAND
GC	CLAYEY GRAVELS, CLAYEY GRAVEL WITH SAND
SW	WELL GRADED SANDS & SAND WITH GRAVEL
SP	POORLY GRADED SANDS & SANDS WITH GRAVEL
SP-SM	POORLY GRADED SAND WITH SILT, SAND WITH SILT & GRAVEL
SM	SILTY SANDS, SILTY SAND WITH GRAVEL
SC	CLAYEY SANDS, CLAYEY SANDS WITH GRAVEL
CH	HIGH PLASTICITY CLAY
CL-ML	SILTY CLAY TO CLAYEY SILT
CL	LOW PLASTICITY CLAY, GRAVELLY-SANDY CLAYS
ML	SILT, GRAVELLY-SANDY SILT
OL	ORGANIC CLAY, SANDY-GRAVELLY ORGANIC SOIL
OH	ORGANIC, HIGH PLASTICITY, SILTY CLAY
PT	PEAT, MUCK, ORGANIC SOILS



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NO.	BY	DATE	REVISION	APP'D.
3.				
2.				
1.				

PROJECT: **WISCONSIN PUBLIC SERVICE CORPORATION**
WESTON DISPOSAL SITE NO. 3 EXPANSION
PLAN OF OPERATION

SHEET TITLE:
ENGINEERING CROSS SECTION 325,300N

DRAWN BY:	LSTORMER	SCALE:	PROJ. NO. 196089.0003
CHECKED BY:	TDH	AS SHOWN	FILE NO. 196089.0003.SHT17-20-XS.dwg
APPROVED BY:	CDM	DATE PRINTED:	**SHEET 20 OF 29**
DATE:	MARCH 2014		

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708 Heartland Trail
Suite 3000
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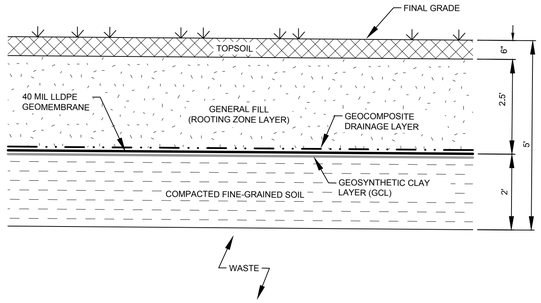
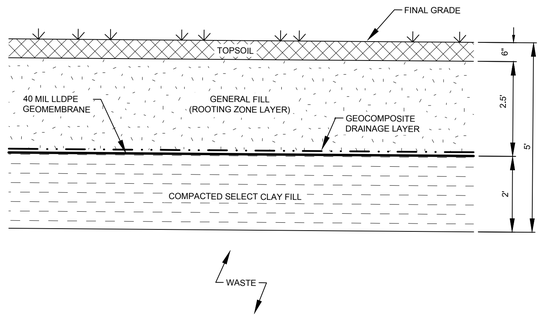
Phone: 608.826.3600

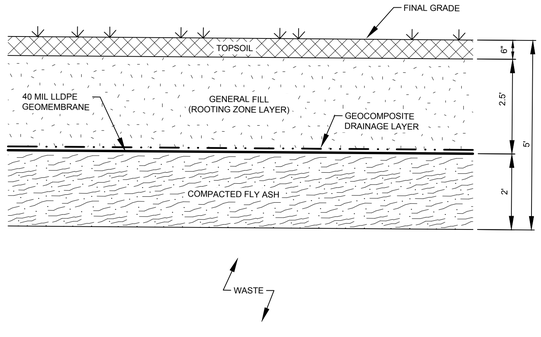
File Location: I:\WPS\Weston Disposal Site\196089\0003 **File Name:** 196089.0003.SHT17-20-XS **Layout Name:** SHEET 20
Plot Information: NONE STORMER, LARRY ---- ANSI A (8.50 X 11.00 INCHES) March 13, 2014 0.3869 K:\AUTOCAD_CONFIG\PLOTTERS\PLOT STYLES\RMT COLOR.CTB



OPTION 1



OPTION 2



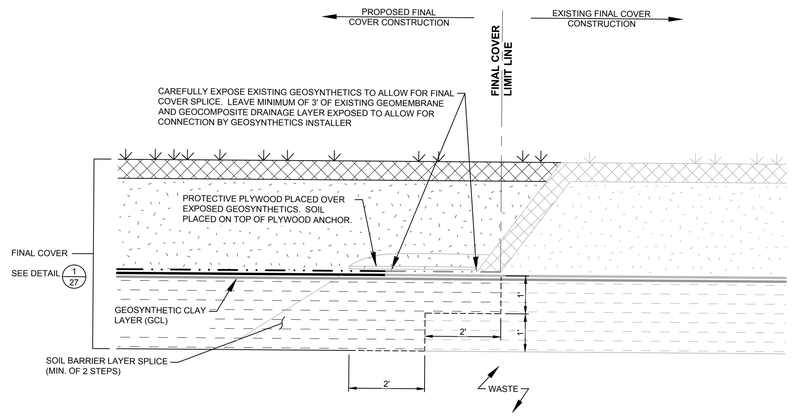
OPTION 3

NOTES:

1. 40 MIL LLDPE IS TEXTURED ON 4H:1V SLOPES AND SMOOTH ON 5% SLOPES.
2. A SELECT GRANULAR FILL DRAINAGE LAYER (SAND) MAY BE SUBSTITUTED FOR THE GEOCOMPOSITE DRAINAGE LAYER FOR OPTIONS 1, 2, OR 3. THE SELECT GRANULAR FILL WILL MEET THE REQUIREMENTS OF NR 504.07(6)(a). IF THIS OPTION IS SELECTED, THE THICKNESS OF THE GENERAL FILL (ROOTING ZONE LAYER) WOULD BE REDUCED TO 1.5 FEET.

1/27 FINAL COVER

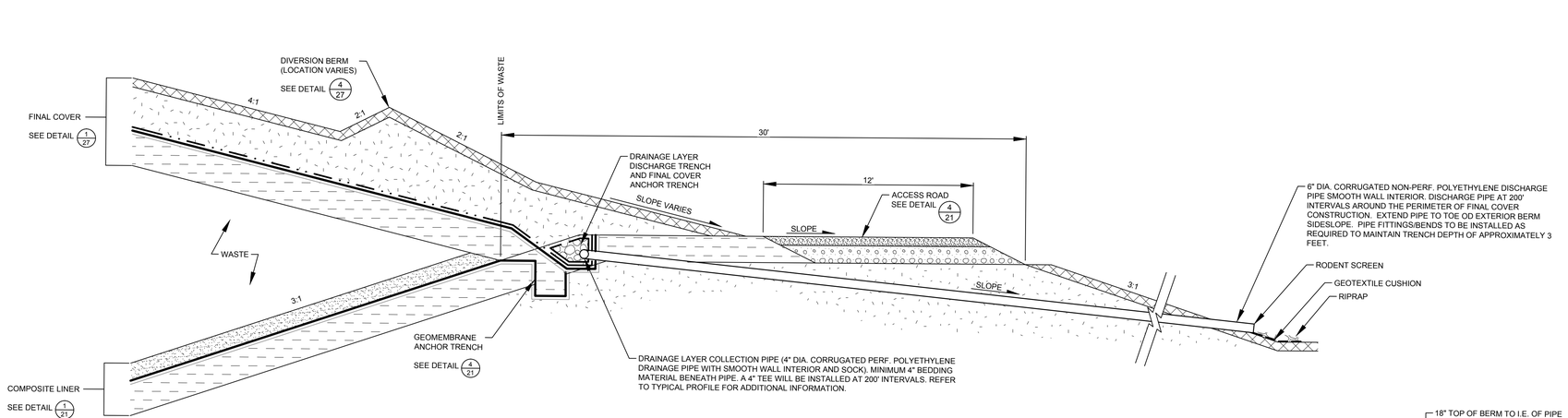
SCALE: 1"=2'



NOTE: SCREENED IMAGE REPRESENTS EXISTING FINAL COVER CONSTRUCTION. BOLD IMAGE REPRESENTS NEW FINAL COVER CONSTRUCTION.

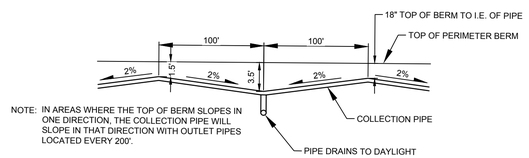
3/27 FINAL COVER SPLICE

SCALE: 1"=2'



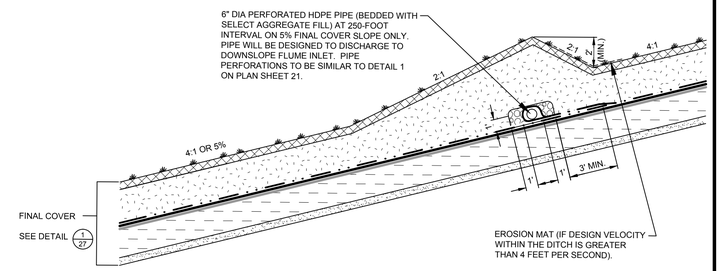
2/27 PERIMETER BERM/FINAL COVER DRAINAGE LAYER DISCHARGE (TYPICAL)

SCALE: 1"=4'



NOTE: IN AREAS WHERE THE TOP OF BERM SLOPES IN ONE DIRECTION, THE COLLECTION PIPE WILL SLOPE IN THAT DIRECTION WITH OUTLET PIPES LOCATED EVERY 200'.

TYPICAL PROFILE OF FINAL COVER DRAINAGE LAYER DISCHARGE AS VIEWED FROM OUTSIDE OF PERIMETER BERM



4/27 FINAL COVER DIVERSION BERM

SCALE: 1"=5'

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NOT FOR CONSTRUCTION

NO.	BY	DATE	REVISION	APP'D.
3.				
2.				
1.				

PROJECT: **WISCONSIN PUBLIC SERVICE CORPORATION**
WESTON DISPOSAL SITE NO. 3 EXPANSION PLAN OF OPERATION

SHEET TITLE: **DETAILS**

DRAWN BY: LSTORMER	SCALE: AS SHOWN	PROJ. NO. 196089.0003	
CHECKED BY: TDH		FILE NO. 196089.0003.SHT27-DT.dwg	
APPROVED BY: CDM	DATE PRINTED:	SHEET 27 OF 29	
DATE: MARCH 2014			

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Phone: 608.826.3600

File Location: J:\WPS\Weston Disposal Site\196089\0003 **File Name:** 196089.0003.SHT27-DT **Layout Name:** SHEET 27
Plot Information: NONE NOLDEN, RANDY ---- ANSI A (8.50 X 11.00 INCHES) March 13, 2014 0.3869 ----